

Constitutive equations

I. Yielding, condition of loading

Yielding criterion is

$$\bar{\sigma} = Y(\bar{\varepsilon}^p). \quad (1)$$

The subsequent behaviour for a stress state at yielding can be characterized as loading, unloading and neutral.

$$\begin{cases} d\bar{\sigma} > 0 & \text{loading} \\ d\bar{\sigma} = 0 & \text{neutral} \\ d\bar{\sigma} < 0 & \text{unloading} \end{cases}, \quad (2)$$

which can be rewritten as

$$\begin{cases} \sigma'_{ij} d\sigma'_{ij} > 0 & \text{loading} \\ \sigma'_{ij} d\sigma'_{ij} = 0 & \text{neutral} \\ \sigma'_{ij} d\sigma'_{ij} < 0 & \text{unloading} \end{cases}, \quad (3)$$

When an element of material is unloaded from a certain plastic state, it recovers elasticity. The elastic behaviour of the material can be characterized by the same Young's modulus E and Poisson's ν ratio in the previous elastic stage. When the element is reloaded along a certain strain-path, yielding will occur again when the equivalent stress $\bar{\sigma}$ reaches $Y(\bar{\varepsilon}^p)$.

Exercise 1

For a material point, starting from a free-state increase the stress through uniaxial tension to σ_Y

$$\begin{pmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \rightarrow \begin{pmatrix} \sigma_Y & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}. \text{ Hold } \sigma_{11} = \sigma_Y \text{ while increase } \sigma_{22} = \sigma \text{ monotonically.}$$

(1) What will be the loading condition for the stress state $\begin{pmatrix} \sigma_Y & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{pmatrix}$ when $\sigma = 0$.

(2) Keep increasing σ , when will the stress state $\begin{pmatrix} \sigma_Y & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{pmatrix}$ goes back from elastic deformation to yielding?

Solution:

(1) The deviatoric stress tensor is $\boldsymbol{\sigma}' = \begin{pmatrix} \frac{2\sigma_Y - \sigma}{3} & 0 & 0 \\ 0 & \frac{2\sigma - \sigma_Y}{3} & 0 \\ 0 & 0 & -\frac{\sigma_Y + \sigma}{3} \end{pmatrix}$

$$\bar{\sigma} = \sqrt{\frac{3}{2} \sigma'_{ij} \sigma'_{ij}}$$

Therefore, the equivalent stress is
$$= \sqrt{\frac{3}{2} \left[\left(\frac{2\sigma_Y - \sigma}{3} \right)^2 + \left(\frac{2\sigma - \sigma_Y}{3} \right)^2 + \left(-\frac{\sigma_Y + \sigma}{3} \right)^2 \right]}$$

$$= \sqrt{\sigma_Y^2 + \sigma^2 - \sigma\sigma_Y}$$

We have
$$\frac{d\bar{\sigma}}{d\sigma} = \frac{2\sigma - \sigma_Y}{2\sqrt{\sigma_Y^2 + \sigma^2 - \sigma\sigma_Y}}.$$

When $\sigma = 0$, $\frac{d\bar{\sigma}}{d\sigma} < 0$, unloading.

(2) Let $\bar{\sigma} = \sqrt{\sigma_Y^2 + \sigma^2 - \sigma\sigma_Y} = \sigma_Y$, one has $\sigma = \sigma_Y$

II. Deformation

In this section, we consider the elastic-plastic behaviour the solid is characterized by its elastic constants E , ν and by the yield stress $Y(\bar{\varepsilon}^p)$ as a function of accumulated plastic strain $\bar{\varepsilon}^p$ and its slope $h = \frac{dY}{d\bar{\varepsilon}^p}$.

A deformation state is described by the stress σ_{ij} , strain ε_{ij} , and the equivalent plastic strain $\bar{\varepsilon}^p$. Under a given stress increment $d\sigma_{ij}$, the strain increment $d\varepsilon_{ij}$ can be decomposed into two parts,

$$d\varepsilon_{ij} = d\varepsilon_{ij}^e + d\varepsilon_{ij}^p. \quad (4)$$

The elastic part of the strain is related to stress using the linear elastic equations as

$$d\varepsilon_{ij} = \frac{1+\nu}{E} d\sigma_{ij} - \frac{\nu}{E} d\sigma_{kk} \delta_{ij}. \quad (5)$$

If the deformation is in elastic stage, i.e.,

$$\bar{\sigma} < Y(\bar{\varepsilon}^p), \quad (6)$$

there is no strain increase, $d\varepsilon_{ij}^p = 0$.

If the current stage is on the yielding surface, the plastic strain increment $d\varepsilon_{ij}^p$ depends on the loading or unloading condition.

$$\begin{cases} d\varepsilon_{ij}^p = \sigma'_{ij} d\lambda & \text{if } \sigma'_{ij} d\sigma_{ij} \geq 0 \\ d\varepsilon_{ij}^p = 0 & \text{if } \sigma'_{ij} d\sigma_{ij} < 0 \end{cases}, \quad (7)$$

where

$$d\lambda = \frac{3}{2} \frac{d\bar{\varepsilon}^p}{\bar{\sigma}}. \quad (8)$$

Therefore, for the loading case

$$\begin{aligned}d\varepsilon_{ij}^p &= \sigma'_{ij} d\lambda \\ &= \frac{3}{2} \frac{\sigma'_{ij}}{\bar{\sigma}} d\bar{\varepsilon}^p \\ &= \frac{3}{2} \frac{\sigma'_{ij}}{h\bar{\sigma}} d\bar{\sigma} \\ &= \frac{9}{4} \frac{1}{h} \frac{\sigma'_{ij}}{\bar{\sigma}^2} \sigma'_{kl} d\sigma'_{kl}\end{aligned}\tag{9}$$

Exercise 2

Following Exercise 1, keep increasing σ , what is the strain tensor when the stress state reaches

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_Y & 0 & 0 \\ 0 & 2\sigma_Y & 0 \\ 0 & 0 & 0 \end{pmatrix} ?$$